

# 2-D Hydraulic Modeling

## Theory & Practice

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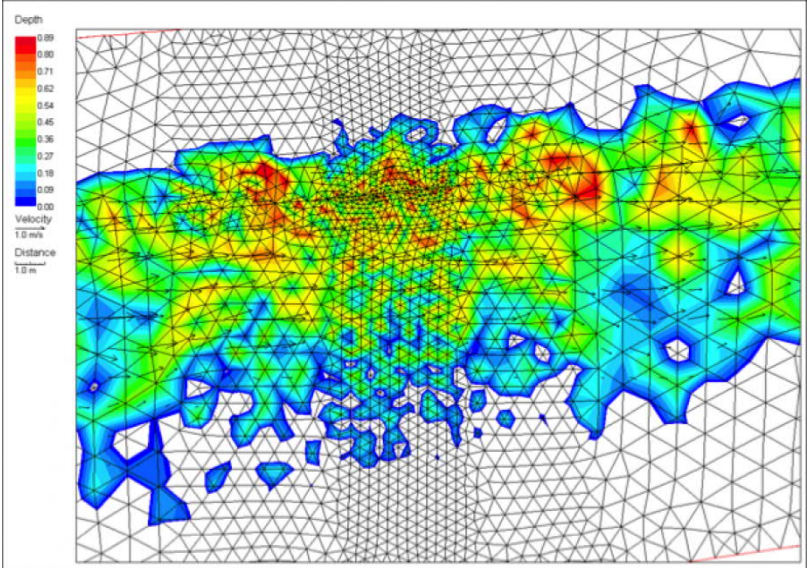
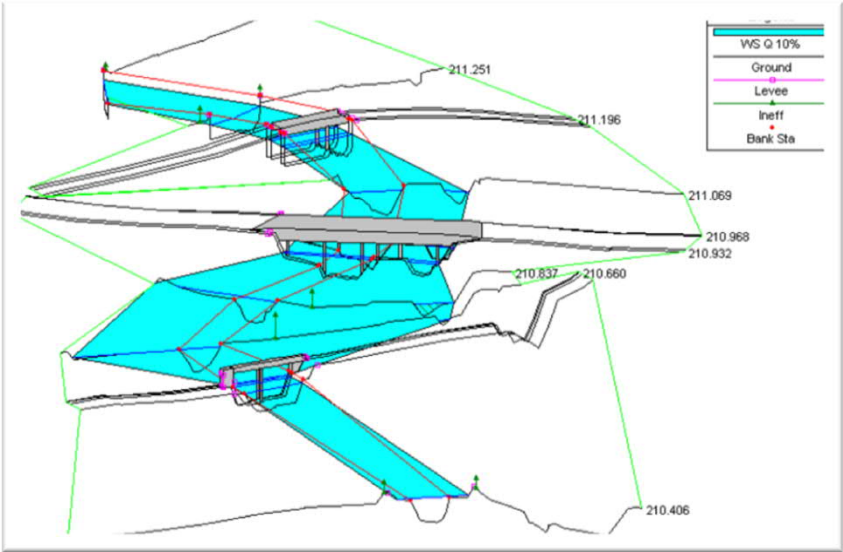
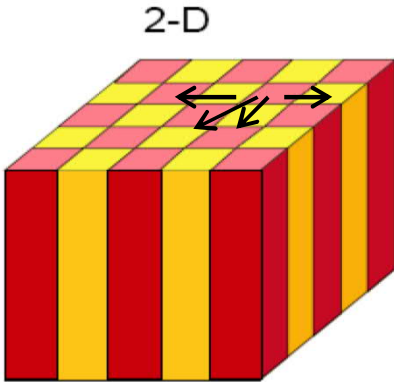
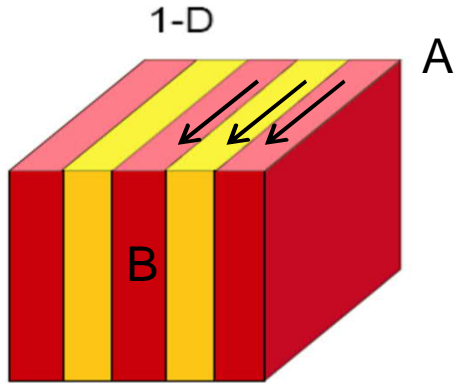
# Presentation Outline

- \* 1-D vs. 2-D modeling
- \* Theory of 2-D simulation
- \* Commonly used and accepted software
- \* Real-life applications
- \* Conclusions
- \* Discussion

# 1-D vs. 2-D

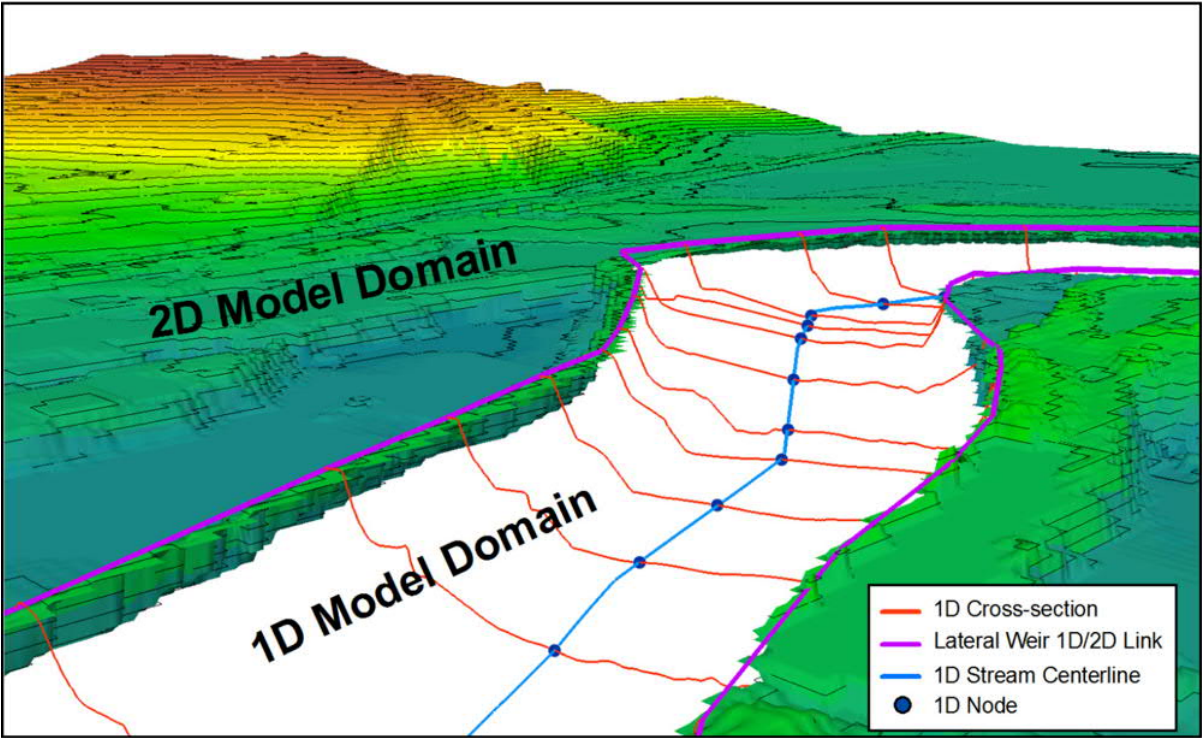


# 1-D vs. 2-D



# 1-D vs. 2-D

## Hybrid Models





# 1-D vs. 2-D

## \* When will a 1D model be suitable?

1. Locations where flow isn't required to 'spread' significantly (flow maintains primarily uni-directional flow patterns).
2. Well-defined channel/overbank systems (channel is bounded by steep slopes, constricting the lateral expansion of flows).
3. Simply-connected floodplains where flow in main channel is well connected to flow in the overbank and that flow in both is primarily uni-directional in nature.
4. When elevation data of only limited quality/quantity are available.

## \* When is a 2D model usually preferable?

- 1: Should be used when the engineer has great difficulty visualizing the flow patterns
- 2: Anywhere flow is expected to spread, such as urbanized areas, wide floodplains, downstream of levee/dam breaks, wetland studies, lake or estuary studies and alluvial fans

# 1-D Vs. 2-D

## 1-D

- \* Can overestimate depth and velocity due to 1D assumptions
- \* Requires engineering judgement
- \* Can't model dispersive flow

## 2-D

- \* Simple overland model construction
- \* More informative dynamic mapping
- \* Limited on hydraulic structures
- \* Heavy reliance on terrain quality

# Other Considerations

- \* **Run times.** If your 2D area is very large and you have relatively small cells (i.e. a lot of cells), then run times can be long. By a lot of cells, say 100,000 to 1 million or more. Making your model 2D in areas where you need detail and 1D everywhere else can help solve this problem.
- \* **Output.** Getting output from 2D areas is a bit more cumbersome and limited. Still, you can get quite a bit of information out of your 2D areas, it just might take more time.
- \* **Learning curve.** Being new to 2D modeling, there will be some additional time for learning how to do 2D modeling.
- \* **End-user may not be okay with it.** Make sure the 2D modeling is acceptable to the end user. There is generally a perception that 2D modeling is more expensive. This is not (should not) always be the case.



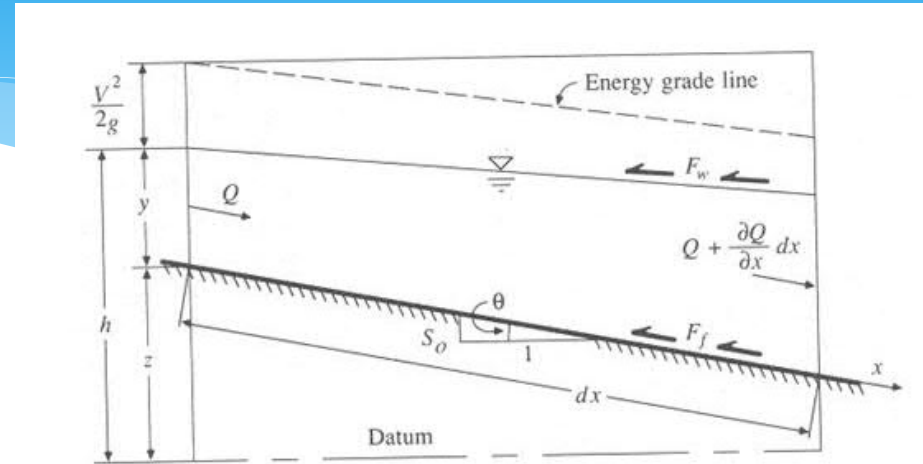
Theory

# Saint-Venant Equation (Continuity Equation)

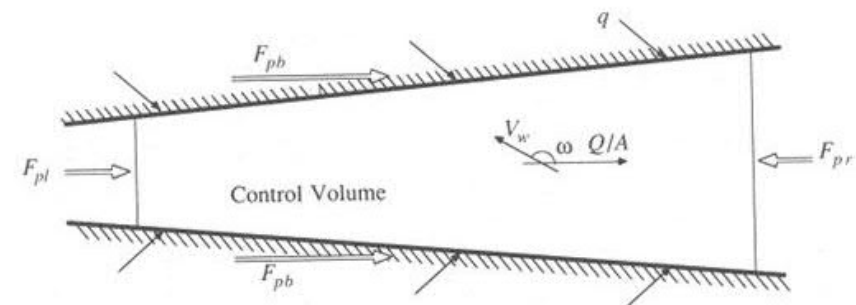
$$\frac{\partial Q}{\partial x} + \frac{\partial A}{\partial t} = 0$$

Change in  
flow

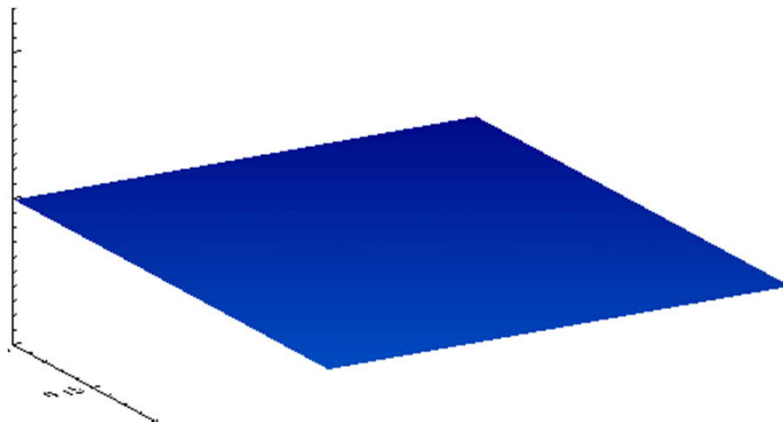
Change in  
storage



Elevation View



Plan View



[https://en.wikipedia.org/wiki/Shallow\\_water\\_equations#/media/File:Shallow\\_water\\_waves.gif](https://en.wikipedia.org/wiki/Shallow_water_equations#/media/File:Shallow_water_waves.gif)

# Saint-Venant Equation (Momentum Equation)

Local  
acceleration

Convective  
acceleration

Pressure  
force

Gravity  
force

Friction  
force

$$\frac{\partial V}{\partial t} + V \frac{\partial V}{\partial x} + g \frac{\partial y}{\partial x} - g(S_0 - S_f) = 0$$

Kinematic wave

Diffusion wave

Dynamic wave

Steady, uniform flow

Steady, non-uniform flow

Unsteady, non-uniform flow

[https://en.wikipedia.org/wiki/Shallow\\_water\\_equations](https://en.wikipedia.org/wiki/Shallow_water_equations)

Software

# 2-D Hydraulic Software

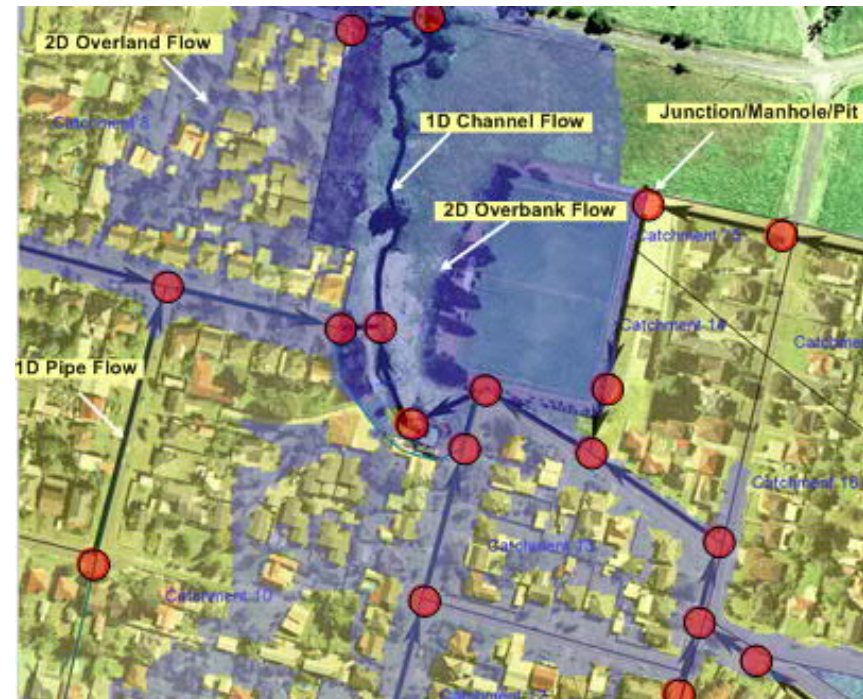


- \* FLO-2D
  - \* FEMA approved
  - \* Free of charge(Basic Model)
  - \* Combined 1-D and 2-D
  - \* Storm drain system
  - \* Scour analysis
  - \* Dam and levee breach
  - \* Mud flow

# 2-D Hydraulic Software



- \* XP-SWMM 2D
  - \* FEMA approved
  - \* Hydrologic Model
  - \* Combined 1-D, 2-D, and Storm drain system
  - \* Plume and sediment transport
  - \* Real-time control





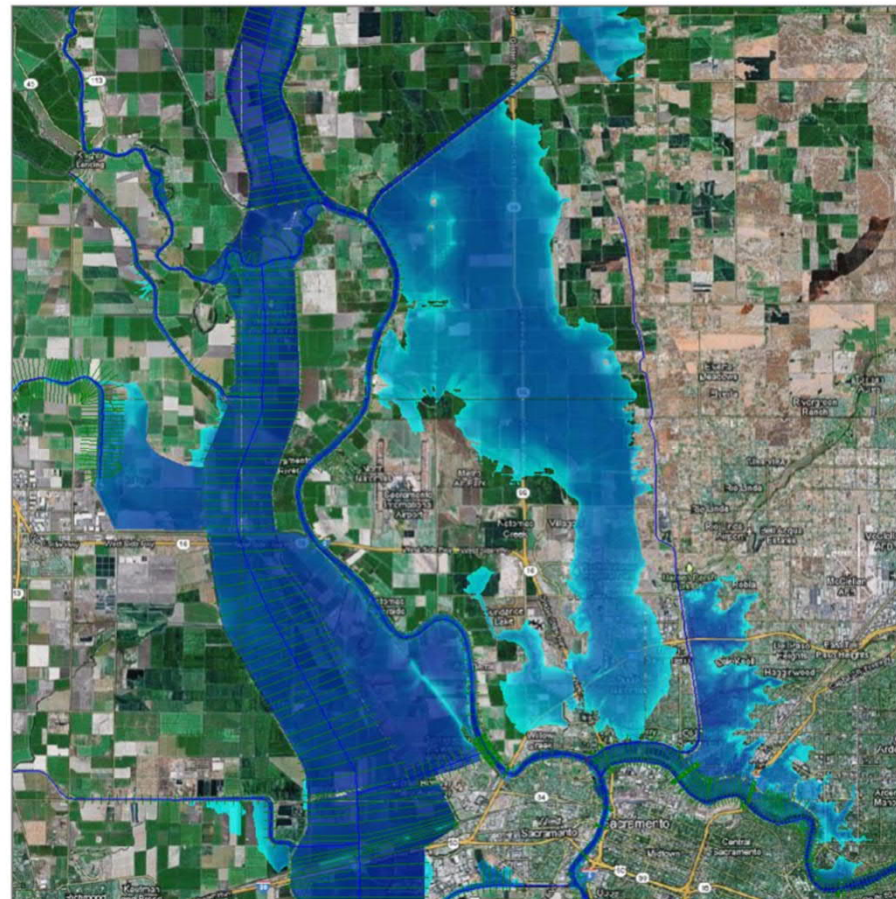
# 2-D Hydraulic Software

- \* HEC-RAS 5.0.3 2D



US Army Corps of Engineers

- \* FEMA approved
- \* Free of charge
- \* Combined 1-D and 2-D
- \* Dam and levee breach
- \* Plume and sediment transport
- \* RAS Mapper

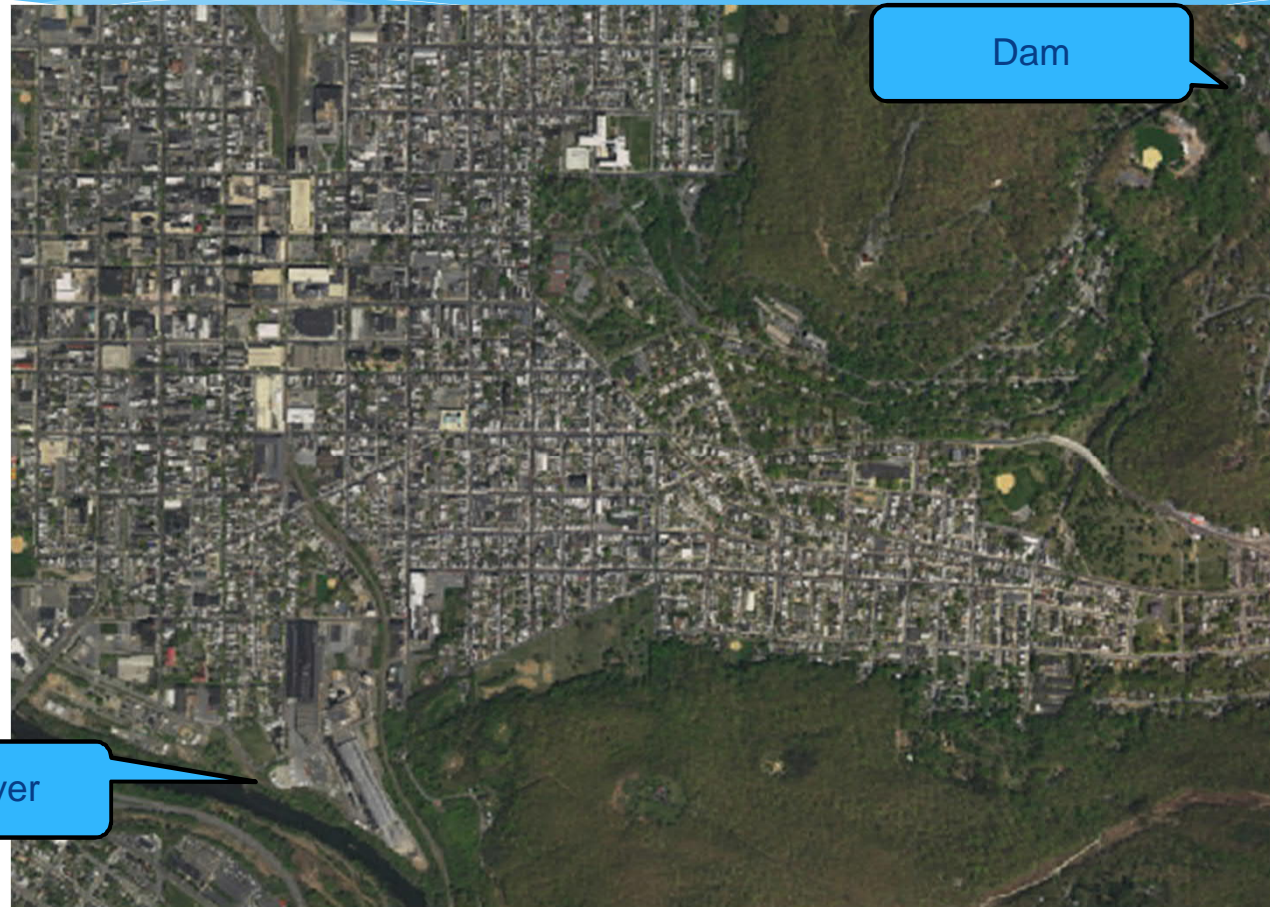


# Applications

# 2D Modeling for Dam Break Analysis

Dam height 15 ft  
Watershed 0.6 sq.mi  
PMF flow 3,100 cfs

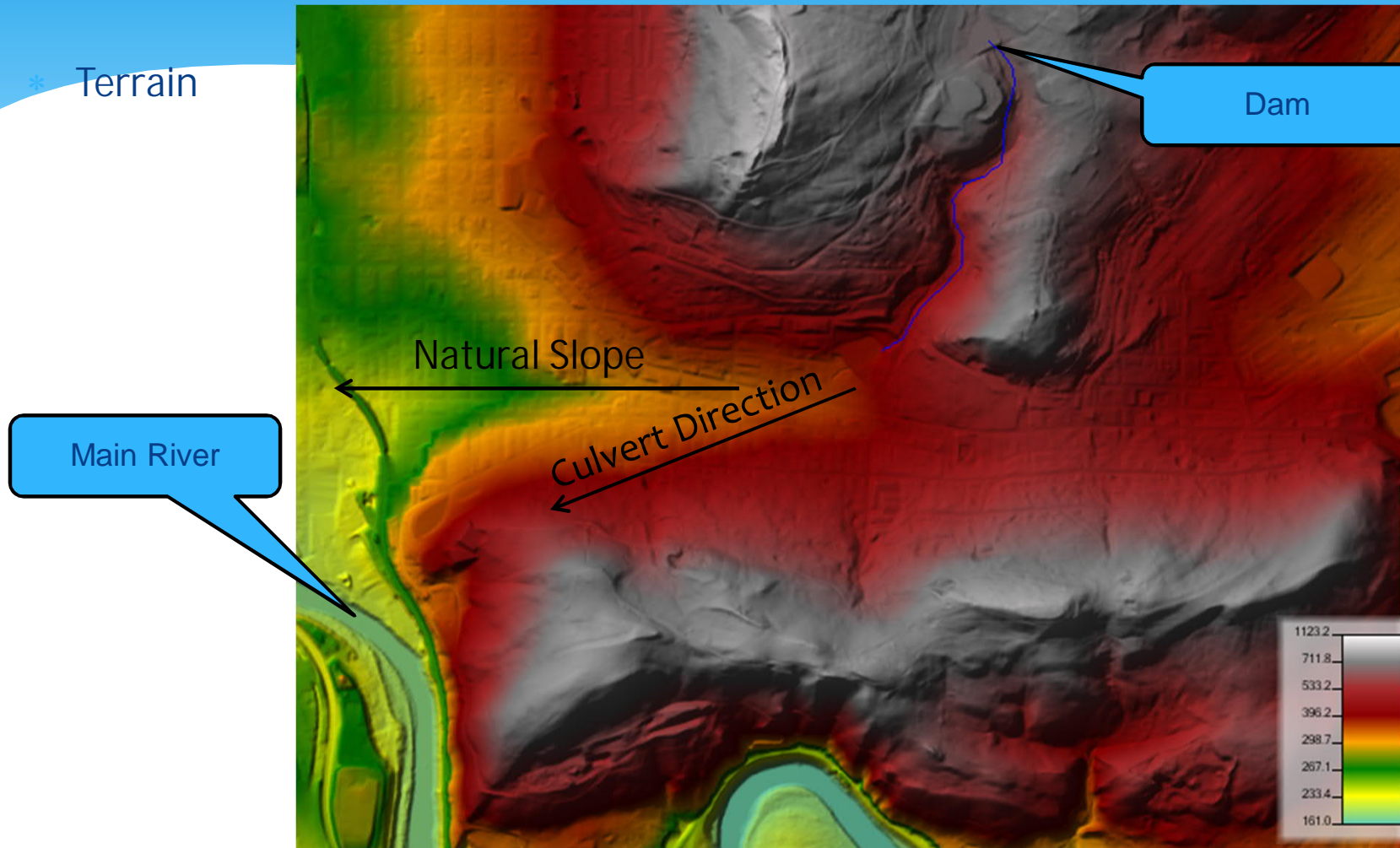
Flow goes underground  
within city limits



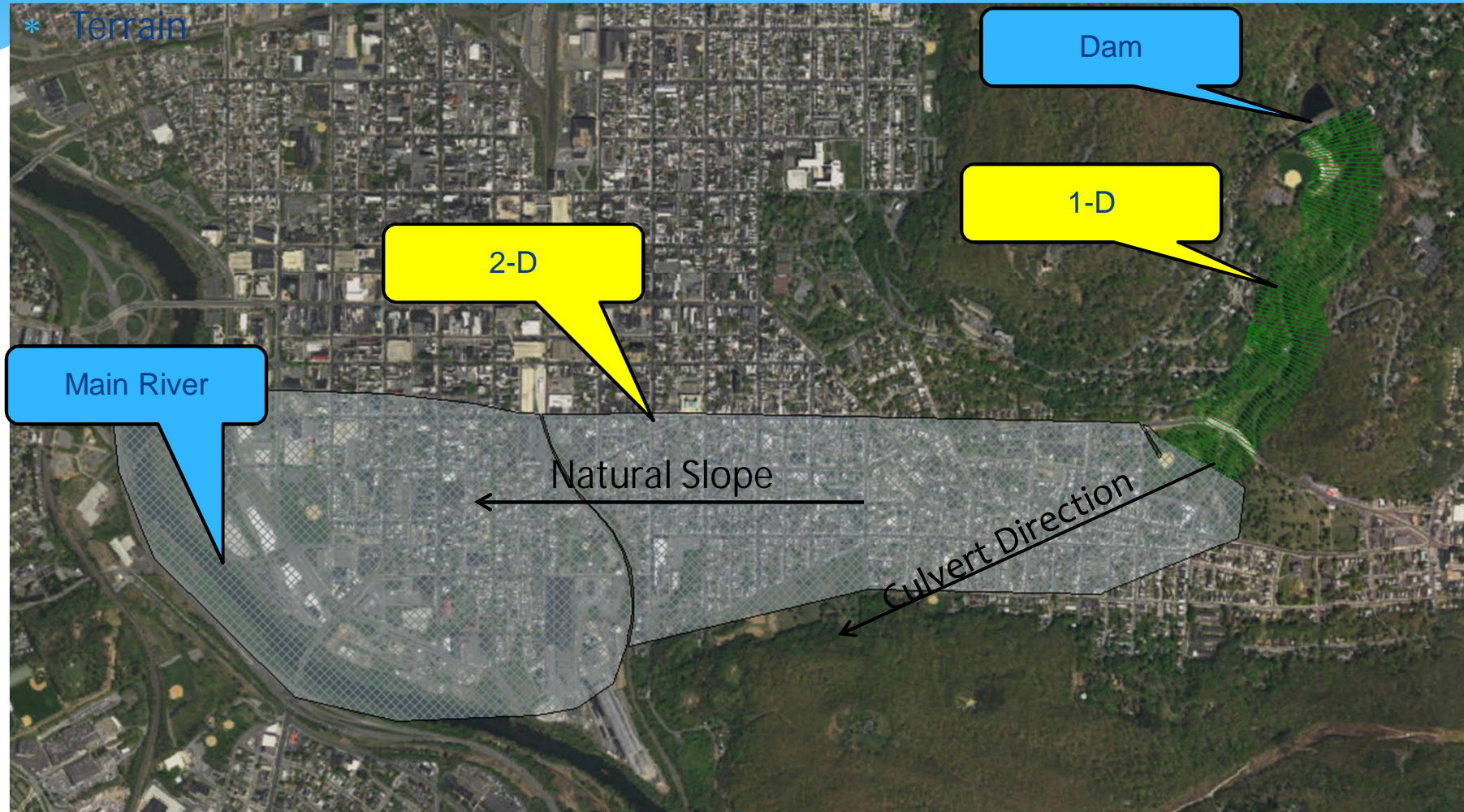


# 2D Modeling for Dam Break Analysis

\* Terrain



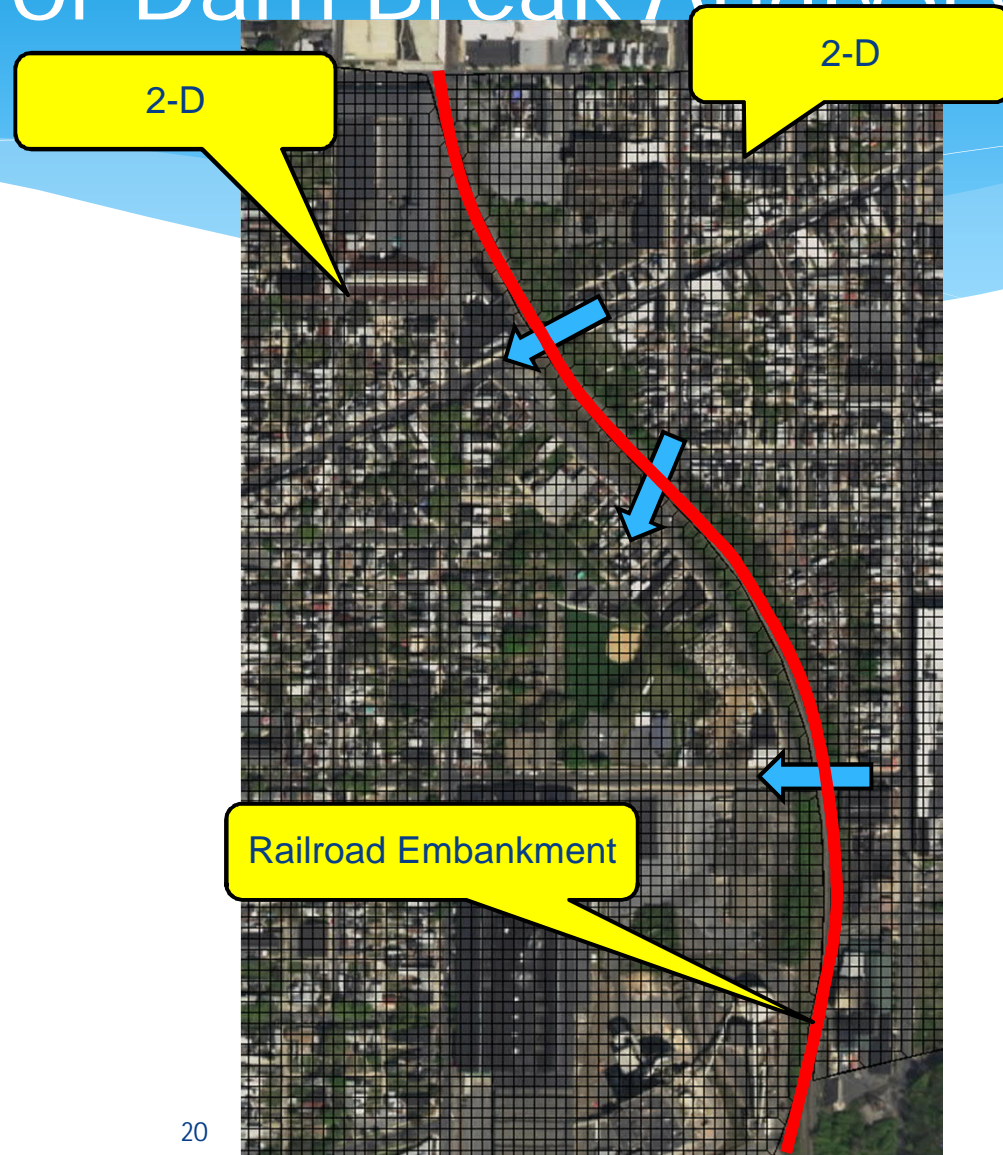
# 2D Modeling for Dam Break Analysis





# 2D Modeling for Dam Break Analysis

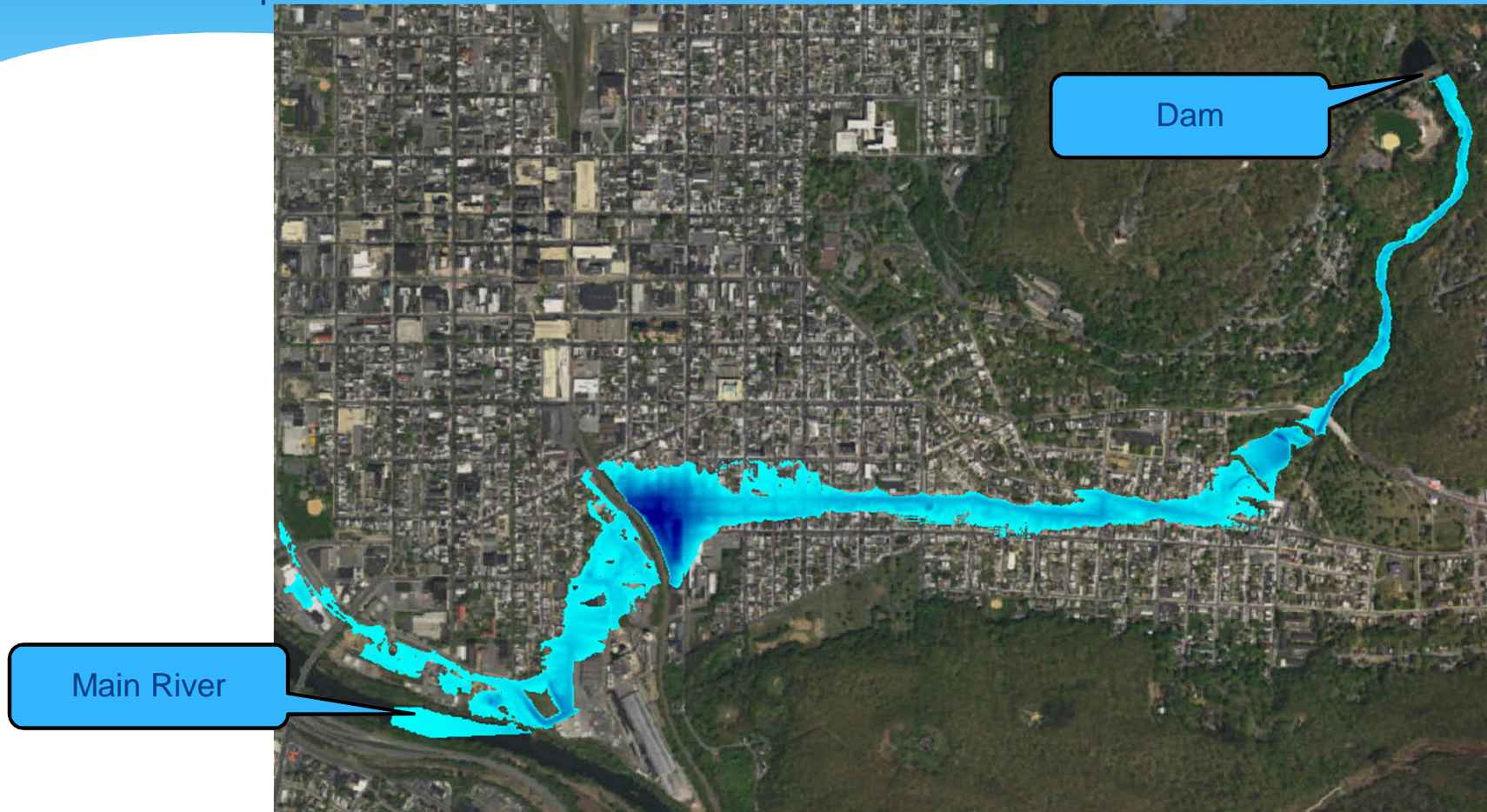
- \* 2D Areas Connectors





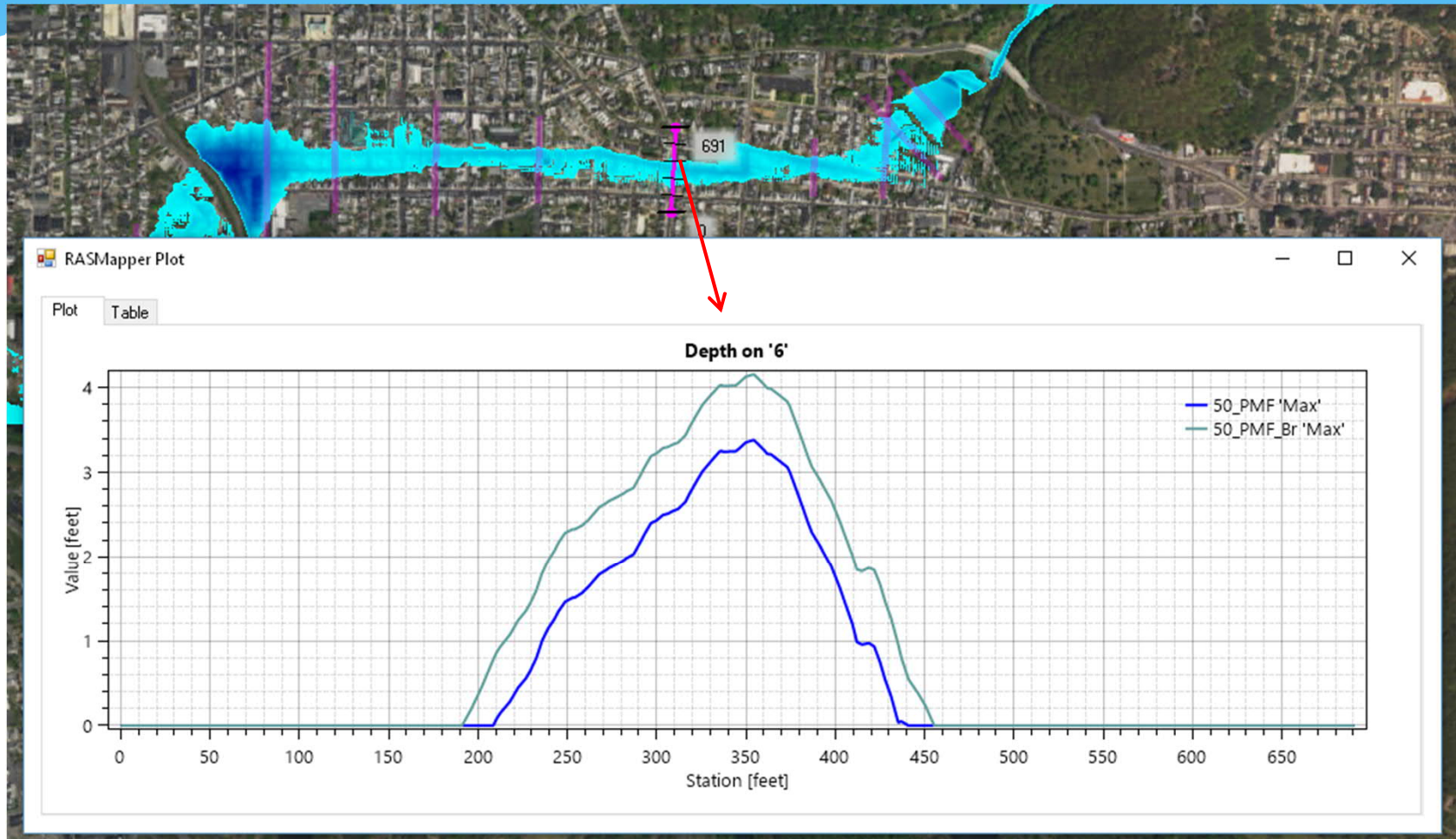
# 2D Modeling for Dam Break Analysis

Maximum Depth



# 2D Modeling for Dam Break Analysis

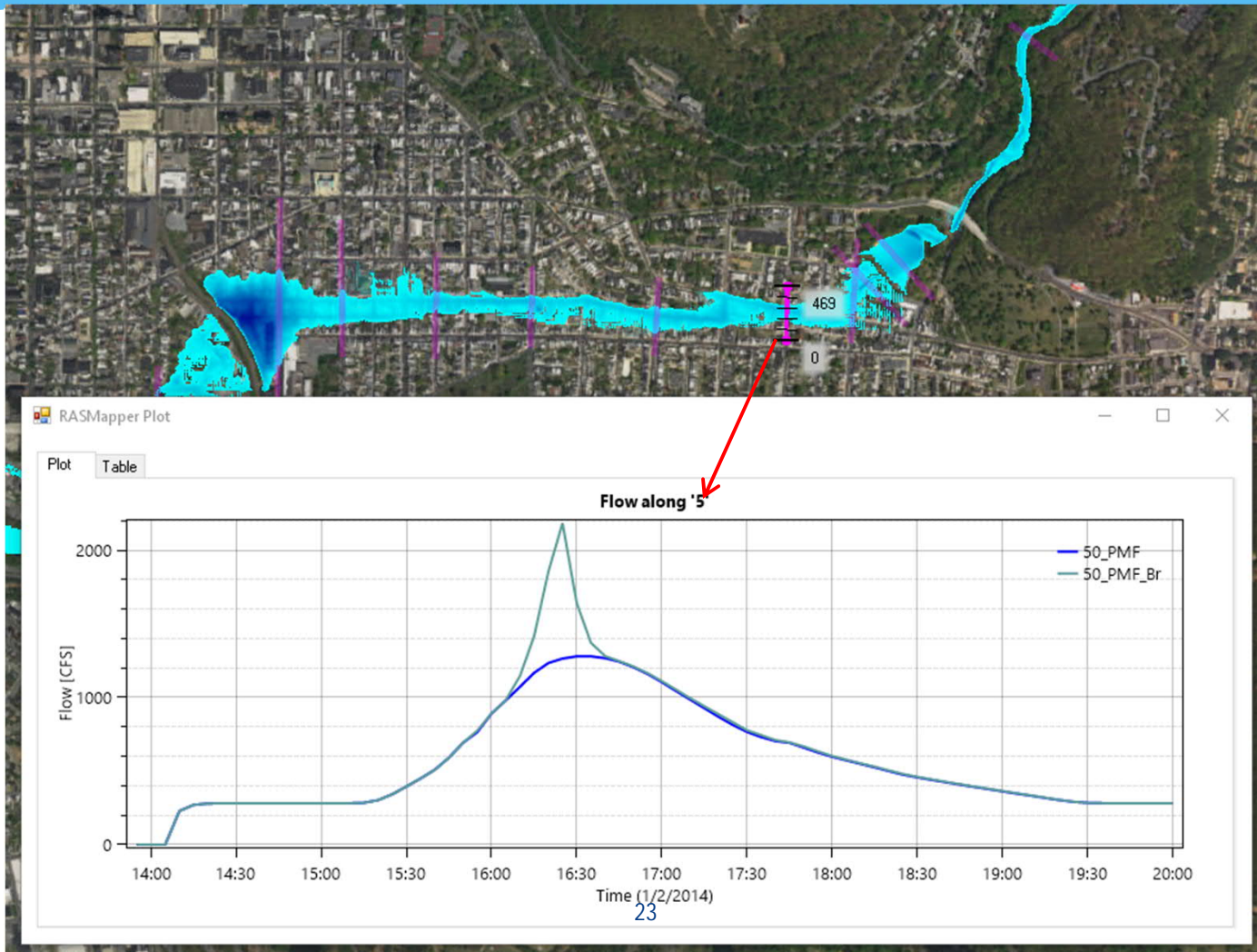
## Profile Depth Plot



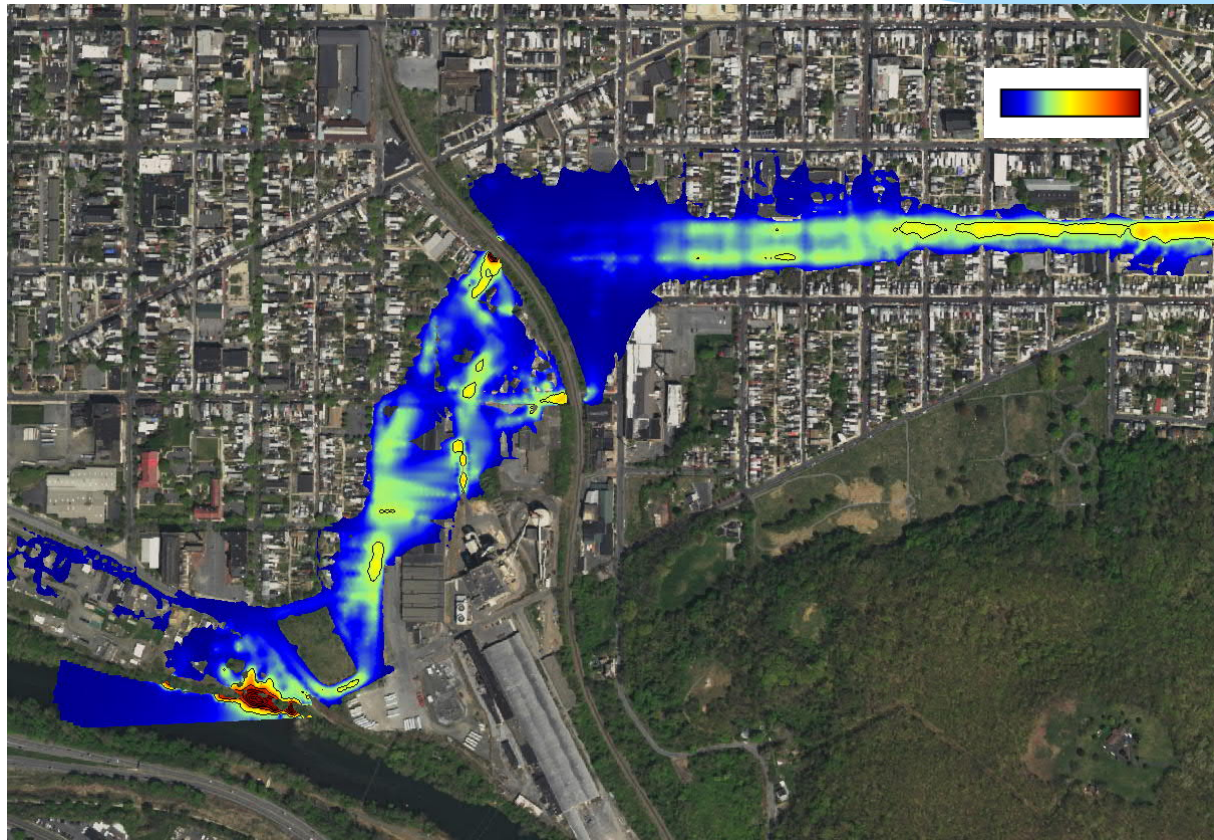


# 2D Modeling for Dam Break Analysis

## Time Series Flow Plot



# 2D Modeling for Dam Break Analysis– velocity grid



# Conclusions

- \* 2D or 1D/2D models offer significant gains
  - \* in accuracy of flood modeling (flow path, depressions, diversions)
  - \* risk and flood impact predictions
  - \* in stakeholder understanding and acceptance
- \* Slow in comparison to 1D only
- \* Models need to be
  - \* Calibrated where possible
  - \* Quality Controlled: Garbage In / Garbage Out



# Discussion

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